



## ► Report on:

Member Selection and  
Structural Design

Volume 7, Number 5

# Design Capacities for Sheet Metal Screws in Lightweight Steel Framing Applications

This Technical Bulletin provided the factored resistance of connections made with sheet metal screws calculated in accordance with CAN/CSA-S136-16 North American Specification for the Design of Cold Formed Steel Structural Members. This bulletin is intended as a guide to help simplify the design of these connections.<sup>1</sup>

### Material Properties

Calculations were based on the mechanical properties of the lightweight steel framing components listed in Table 1, and the properties of the screws listed in Table 2.

Table 1: Design Thickness and Mechanical Properties of LSF Components

Gauge Number	Thickness Designation (mils)	Design Thickness (mm)	Strengths	
			Yield, $F_y$ (MPa)	Ultimate, $F_u$ (MPa)
20	33	0.879	230	310
18	43	1.146	230	310
16	54	1.438	345	450
14	68	1.811	345	450
12	97	2.583	345	450

Table 2: Nominal Diameter and Strength of Screws<sup>2</sup>

Number Designation for Screw	Nominal Diameter (mm)	Nominal Shear Strength, $P_{nvs}$ (kN)	Nominal Tension Strength, $P_{nts}$ (kN)
#6 - 20	3.56	3.34	5.72
#8 - 18	4.06	4.45	6.87
#10 - 16	4.83	6.23	8.61
#12 - 14	5.33	8.90	12.36
¼ - 14	6.35	11.57	18.06

Canadian Sheet Steel  
Building Institute  
652 Bishop St. N., Unit 2A  
Cambridge, Ontario N3H 4V6  
Tel.: (519) 650-1285  
Fax: (519) 650-8081  
Web Site: [www.cssbi.ca](http://www.cssbi.ca)

1 While the material is believed to be technically correct and in accordance with recognized practice at the time of publication, it does not obviate the need to determine its suitability for a given situation. Neither the Canadian Sheet Steel Building Institute nor its Members warrant or assume any liability for the suitability of the material for any general or particular purpose.

2 These values were taken from the ITW Buildex 2010/2011 product catalogue for TEKS self-drilling self-tapping screws and may not be appropriate for other screw types or products from other screw manufacturers.

## ▶ Report on:

Member Selection and  
Structural Design

### Factored Resistance of Screwed Connections

The factored resistance of screwed connections is a function of the failure type, screw size and sheet properties. Listed in Table 3 are the factored resistance values for the various limits. The minimum value of the controlling limit state will govern.

### Minimum Edge and End Distance (S136 Clause J4.2)

The distance from the center of a fastener to the edge or end of any part shall not be less than 1.5d.

### Design Equations for Shear (S136 Clause J4.3)

*Connection Shear Limited by Tilting and Bearing (S136 Clause J4.3.1):*

For  $t_2/t_1 \leq 1.0$ ,  $P_{nv}$  equals the smallest of;

$$P_{nv} = 4.2(t_2^3 d)^{1/2} F_{u2}$$

$$P_{nv} = 2.7 t_1 d F_{u1}$$

$$P_{nv} = 2.7 t_2 d F_{u2}$$

For  $t_2/t_1 \geq 2.5$ ,  $P_{nv}$  equals the smallest of;

$$P_{nv} = 2.7 t_1 d F_{u1}$$

$$P_{nv} = 2.7 t_2 d F_{u2}$$

For  $t_2/t_1$  values between 1.0 and 2.5,  $P_{nv}$  is determined through linear interpolation

*Shear in Screws (S136 Clause J4.3.2):*

The nominal shear resistance of the screw is taken as  $P_{nvs}$ .

### Design Equations for Tension (S136 Clause J4.4)

*Pull-out (S136 Clause J4.4.1):*

$$P_{not} = 0.85 t_c d F_{u2}$$

*Pull-over (S136 Clause J4.4.2):*

$$P_{nov} = 1.5 t_1 d'_w F_{u1}$$

*Tension in Screws (S136 Clause J4.4.3):*

The nominal tensile resistance of the screw is taken as  $P_{nts}$ .

### Combined Shear and Pull-Over (S136 Clause J4.5.1)

For connections subjected to a combination of both shear and tension forces, the following interaction equation applies.

$$\frac{\bar{V}}{P_{nv}} + 0.71 \frac{\bar{T}}{P_{nov}} \leq 1.10 \Phi$$

where,  $\Phi = 0.55$

The shear/pull-over interaction equation is valid for connections that meet the following limits:

- (1)  $0.724 \text{ mm} \leq t_1 \leq 1.13 \text{ mm}$
- (2) #12 and #14 self-drilling screws with or without washers
- (3)  $d_w \leq 19.1 \text{ mm}$
- (4)  $F_{u1} \leq 483 \text{ MPa}$
- (5)  $t_2/t_1 \geq 2.5$
- (6) washer dimensions limitations of Section J4.4 apply

For eccentrically loaded connections that produce a non-uniform pull-over force on the fastener, the nominal pull-over resistance shall be taken as 50% of  $P_{nov}$ .



► **Report on:**

Member Selection  
and Structural Design

**Combined Shear and Pull-Out (S136 Clause J4.5.2)**

For connections subjected to a combination of both shear and pull-out forces, the following interaction equation applies.

$$\frac{\bar{V}}{P_{nv}} + \frac{\bar{T}}{P_{not}} \leq 1.15 \Phi$$

where,  $\Phi = 0.50$

The shear/pull-out interaction equation is valid for connections that meet the following limits:

- (1)  $0.754 \text{ mm} \leq t_1 \leq 1.84 \text{ mm}$
- (2) #8, #10, #12 or #14 self-drilling screws with or without washers
- (3)  $F_{u2} \leq 834 \text{ MPa}$
- (4)  $1.0 \leq F_u/F_y \leq 1.62$

**Combined Shear and Tension (S136 Clause J4.5.3)**

For connections subjected to a combination of both shear and tension forces, the following interaction equation applies.

$$\frac{\bar{V}}{P_{nvs}} + \frac{\bar{T}}{P_{nts}} \leq 1.3 \Phi$$

where,  $\Phi = 0.40$

**Rupture (S136 Clause J6)**

The other failure mode that must be considered is the block tear-out of a group of fasteners.

**Symbols**

$d$	=	Nominal screw diameter
$d'_w$	=	Effective pull-over resistance diameter
$F_{u1}$	=	Tensile strength of member in contact with screw head
$F_{u2}$	=	Tensile strength of member not in contact with screw head
$P_{nov}$	=	Nominal pull-over resistance per screw
$P_{nvs}$	=	Nominal shear resistance of screw as reported by manufacturer or determined by independent laboratory testing
$P_{nts}$	=	Nominal tension resistance of screw as reported by manufacturer or determined by independent laboratory testing
$\bar{V}$	=	$V_f$ = Factored shear force in connection
$t_1$	=	Thickness of member in contact with screw head
$t_2$	=	Thickness of member not in contact with screw head
$t_c$	=	Lesser of depth of penetration and thickness $t_2$
$\bar{T}$	=	$T_f$ = Factored tensile force in connection



## Report on:

Member Selection  
and Structural Design

**Table 3: Factored Resistances of Screwed Connections (kN)**

Using the Tables:

For shear loading, the lesser of  $\Phi P_{nvs}$  or  $\Phi P_{nv}$  governs.

For tension loading the lesser of  $\Phi P_{nts}$ ,  $\Phi P_{not}$  or  $\Phi P_{nov}$  governs.

Check  $P_{nvs}$  and  $P_{nts}$  for different screw types or manufacturer.

Interaction equations also need to be checked where there are combined forces.

		$\Phi P_{nvs} = 1.34 \text{ kN}$					$\Phi P_{nts} = 2.29 \text{ kN}$					$\Phi = 0.40$				
		Shear ( $\Phi P_{nv}$ )					Tension									
							Pull-Out ( $\Phi P_{not}$ )					Pull-Over ( $\Phi P_{nov}$ )*				
$t_1 \backslash t_2$		20	18	16	14	12	20	18	16	14	12	20	18	16	14	12
20		0.810	1.05	1.05	1.05	1.05	0.330	0.430	0.783	0.986	1.41	1.30	1.30	1.30	1.30	1.30
18		0.810	1.21	1.37	1.37	1.37	0.330	0.430	0.783	0.986	1.41	1.69	1.69	1.69	1.69	1.69
16		0.810	1.21	2.46	2.49	2.49	0.330	0.430	0.783	0.986	1.41	3.08	3.08	3.08	3.08	3.08
14		0.810	1.21	2.46	3.13	3.13	0.330	0.430	0.783	0.986	1.41	3.88	3.88	3.88	3.88	3.88
12		0.810	1.21	2.46	3.13	4.47	0.330	0.430	0.783	0.986	1.41	5.54	5.54	5.54	5.54	5.54

		$\Phi P_{nvs} = 1.78 \text{ kN}$					$\Phi P_{nts} = 2.75 \text{ kN}$					$\Phi = 0.40$				
		Shear ( $\Phi P_{nv}$ )					Tension									
							Pull-Out ( $P_{not}$ )					Pull-Over ( $P_{nov}$ )*				
$t_1 \backslash t_2$		20	18	16	14	12	20	18	16	14	12	20	18	16	14	12
20		0.865	1.19	1.19	1.19	1.19	0.376	0.490	0.893	1.12	1.60	1.30	1.30	1.30	1.30	1.30
18		0.865	1.29	1.56	1.56	1.56	0.376	0.490	0.893	1.12	1.60	1.69	1.69	1.69	1.69	1.69
16		0.865	1.29	2.63	2.84	2.84	0.376	0.490	0.893	1.12	1.60	3.08	3.08	3.08	3.08	3.08
14		0.865	1.29	2.63	3.57	3.57	0.376	0.490	0.893	1.12	1.60	3.88	3.88	3.88	3.88	3.88
12		0.865	1.29	2.63	3.57	5.10	0.376	0.490	0.893	1.12	1.60	5.54	5.54	5.54	5.54	5.54

		$\Phi P_{nvs} = 2.49 \text{ kN}$					$\Phi P_{nts} = 3.44 \text{ kN}$					$\Phi = 0.40$				
		Shear ( $\Phi P_{nv}$ )					Tension									
							Pull-Out ( $P_{not}$ )					Pull-Over ( $P_{nov}$ )*				
$t_1 \backslash t_2$		20	18	16	14	12	20	18	16	14	12	20	18	16	14	12
20		0.943	1.41	1.42	1.42	1.42	0.447	0.583	1.06	1.34	1.91	1.30	1.30	1.30	1.30	1.30
18		0.943	1.40	1.85	1.85	1.85	0.447	0.583	1.06	1.34	1.91	1.69	1.69	1.69	1.69	1.69
16		0.943	1.40	2.87	3.38	3.38	0.447	0.583	1.06	1.34	1.91	3.08	3.08	3.08	3.08	3.08
14		0.943	1.40	2.87	4.05	4.25	0.447	0.583	1.06	1.34	1.91	3.88	3.88	3.88	3.88	3.88
12		0.943	1.40	2.87	4.05	6.06	0.447	0.583	1.06	1.34	1.91	5.54	5.54	5.54	5.54	5.54



## ► Report on:

Member Selection  
and Structural Design

		#12 Screw $\Phi P_{nvs} = 3.56 \text{ kN}$					$\Phi P_{nts} = 4.94 \text{ kN}$					$\Phi = 0.40$				
		Shear ( $\Phi P_{nv}$ )					Tension									
							Pull-Out ( $P_{not}$ )					Pull-Over ( $P_{nov}$ )*				
$t_1 \backslash t_2$		20	18	16	14	12	20	18	16	14	12	20	18	16	14	12
20		0.991	1.49	1.57	1.57	1.57	0.494	0.644	1.17	1.48	2.11	1.30	1.30	1.30	1.30	1.30
18		0.991	1.48	2.05	2.05	2.05	0.494	0.644	1.17	1.48	2.11	1.69	1.69	1.69	1.69	1.69
16		0.991	1.48	3.01	3.72	3.72	0.494	0.644	1.17	1.48	2.11	3.08	3.08	3.08	3.08	3.08
14		0.991	1.48	3.01	4.25	4.69	0.494	0.644	1.17	1.48	2.11	3.88	3.88	3.88	3.88	3.88
12		0.991	1.48	3.01	4.25	6.69	0.494	0.644	1.17	1.48	2.11	5.54	5.54	5.54	5.54	5.54

		1/4 Screw $\Phi P_{nvs} = 4.63 \text{ kN}$					$\Phi P_{nts} = 7.22 \text{ kN}$					$\Phi = 0.40$				
		Shear ( $\Phi P_{nv}$ )					Tension									
							Pull-Out ( $P_{not}$ )					Pull-Over ( $P_{nov}$ )*				
$t_1 \backslash t_2$		20	18	16	14	12	20	18	16	14	12	20	18	16	14	12
20		1.08	1.66	1.87	1.87	1.87	0.588	0.767	1.40	1.76	2.51	1.30	1.30	1.30	1.30	1.30
18		1.08	1.61	2.44	2.44	2.44	0.588	0.767	1.40	1.76	2.51	1.69	1.69	1.69	1.69	1.69
16		1.08	1.61	3.29	4.44	4.44	0.588	0.767	1.40	1.76	2.51	3.08	3.08	3.08	3.08	3.08
14		1.08	1.61	3.29	4.64	5.59	0.588	0.767	1.40	1.76	2.51	3.88	3.88	3.88	3.88	3.88
12		1.08	1.61	3.29	4.64	7.91	0.588	0.767	1.40	1.76	2.51	5.54	5.54	5.54	5.54	5.54

\* Tabulated values assume  $d'_w = 7.94 \text{ mm}$ . For  $d'_w$  larger than 7.94 mm, multiply tabulated  $P_{nov}$  values by  $(\text{actual } d'_w) / 7.94$ . The limit of  $d'_w \leq 19.1 \text{ mm}$  also applies.

### For More Information from CSSBI

For more information on sheet steel building products, or to obtain other CSSBI publications, contact the CSSBI at the address shown below or visit the website at [www.cssbi.ca](http://www.cssbi.ca).

